

Behavior of High Strength Concrete (HSC) Reinforced Beam with GFRP Bars Using Plastic Damaged Concrete Model

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Abstract — Reinforced concrete structures consists of two main components and the behavior of these structures influenced by gender and material properties, in order to investigate the behavior of concrete reinforced beam that most important flexural element in reinforced concrete structures, we chosen High Strength Concrete (HSC) for concrete and GFRP and steel bars for reinforcement. For compare the new beam element(high strength concrete and GFRP bars), with general beam (general concrete and steel bars) finite element software ABAQUS used, therefore flexural beam models in this software after simulation and analysis, the results shows that the stress in this elements are reduced.

Keyword — Flexural element, High Strength Concrete, GFRP bars, ABAQUS

1. INTRODUCTION

In these days ,the concrete structures in building industry are research attention require to investigate in this field. So it needs to be applied according to international standards; for this reason the use of new technology seems essential in building industry, there for developments that have occurred in field of material technology, researchers are using their main priority. Based on this newly materials such as that FRP composite and behavior of them that main topic of this paper. Because of the advantages such as resistance to corrosion, high tensile strength, low weight and insulation against electromagnetic waves can be expressed in these materials. so in the first step in this paper to provide a new method of structure design we treat about concrete material, reinforcement bars of the beam that the one the main elements of structure.

2. RESEARCH MODELS

Beams are one of the main elements of building structures and usually can be applied in a different rectangular section in reinforced concrete structures. So in this study the model of the beam is rectangular cross section that identified in table 1.

Table 1. Information of rectangular cross section beam

Height(cm)	Width (cm)	Longitude(cm)
20	15	200

Considering that most of the reinforced concrete structure members are under bending so design and study about under bending concrete sections are primary and most important issues in reinforced concrete section structures.To study the behavior of reinforced concrete structure under bending loads we choices a simple ends beam under a two concentrated load in the thirds of the length figure 1.



Figure 1. Rectangulaer concrete beam diagram

3. MECANICAL PROPERTIES OF MODELS

3.1 Concrete

According to this study, we use a high performance concrete beam so the mechanical properties of concrete is show in table (2).

3.2. Steel bar

The beam that study in this research are reinforced and we use the reinforced concrete beam with steel bars for compare with other simulation models the mechanical of steel bars are shown in table (3, 4).

3.3. GFRP bars

In this research we want to replacing GFRP bars with steel ones, so it is nessecary to know GFRP mechanical properties that this details are in table 5.



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Concrete compressive strength (MPa) (f'_c)	Poission's ratio (\mathcal{G})	Elasticity modulus (kg/m3)	Concrete gravity (N/m3)	Concrete density (kg/m3)	
100	0.2	4.78e9	2450	245	

Table 2. Mechanical properties of high strength concrete

Table 3. Mechanical properties of longitude steel bars

Bar size(mm) Yield stress of bars (kg/m ²)		Poission's ratio	Elasticity modulus (kg/m ³)	
10,16	45722509	0.3	2.054E+11	

Bar size(mm)Yield stress of bars (kg/m²)		Poission's ratio (\mathcal{G})	Elasticity modulus (kg/m ³)	
10	35745864.38	0.3	1.34029E+11	

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Table 5.GFRP bar's properties[3]					
Nominal Diameter	10	16			
mm	10				
Nominal Area	71.26	107.0			
mm^2	/1.20	177.9			
fu	877	724			
MPa	027	124			
Tensile Modulus of Elasticity	46	46			
GPa	40	40			
Ultimate Strain	1 70	1.57			
%	1.79				
Unit Weight /length	0.150	0 4271			
kg / m	0.139	0.4271			
Weight per unit Volume	2221.27	2158			
kg / m3	2231.27	2130			
Mess per unit Volume	227 45	220			
kg / m^3	227.43				
Mess per unit Volume	2221.27	2158.16			
N / m^3	2231.27				

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4. DETAILS OF REAINFORCED CONCRETE BEAM MODEL IN FINITE ELEMENT SOFTWARE

Details of samples beam that study in finite element software (ABAQUS) are collected and gathered in table 6. The letters B, S and G are respectively abbreviation of Beam, Steel and GFRP in this research we use ABAQUS V6.10-1 for simulation and analysis

5. REINFORCED CONCRETE BEAM ANALYSIS

For analysis of samples in ABAQUS software, use step model, through the available analysis method in this software, the only way to check the type of damage and failure in dynamic explicit analysis. Therefore, in this study we use this analysis that graphical results of analysis will reported [1].





In this model that is a control model all of the bars are steel with size of 10 mm. According to stress contour that show in graphical analysis results the maximum tension happened with red color that the maximum tension occurred in this model not seen, for more



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Table 0. Samples Details in the ADAQOS software						
Sample	Concrete compressive strength(f'c)	lateral steel bars spacing(mm)	Compression bars	Tensile bars	d'(mm)	d(mm)
B-S2S2	100	80	2SteelΦ10	2SteelΦ10	175	25
B-G2S2	100	80	2SteelΦ10	2GFRPΦ10	175	25
B-G2G2	100	80	2GFRPΦ10	2GFRPΦ10	175	25
B-S3S2	100	80	2SteelΦ10	3SteelΦ16	175	25
B-G3S2	100	80	2SteelΦ10	3GFRPΦ16	175	25
B-G3G2	100	80	2GFRPΦ10	3GFRPΦ16	175	25

Table 6. Samples Details in the ABAQUS software

information about results of this model analysis use reference[1].



Figure 3. Graphical results of B-G2S2 model

In above model the tension bars are 2 GFRP bar with size 10 mm and the maximum stress in this model has occur in compression bars.



Figure 4. Graphical results of B-G2S2 model

In above model all of the main bars are GFRP, the purpose of this replacement is review the GFRP compression bars area role in reduce the flexural member stress.The maximum stress that occur in this model shows that beam reinforcement tolerate minimum stress

and replacing GFRP bars and increase the compression strength of concrete play adhesive role in stress reduce.



Figure 5. Graphical results of B-S3S2 model

Above model is one of the another control model that the all of the beam bars are steel with increase 2 to 3 bars and tension bars size 10 to 16 mm. Meanwhile we study about increasing the percentage of beam reinforced behavior especially in tension area.



Figure 6. Graphical results of B-G3S2 model

In this model tension area are replacing with GFRP but the compression areas bar are steel.

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Figure 7. Graphical results of B-G3G2 model

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In this model all of the bars are reinforcement with GFRP bars to study the role of compression area bars in improve the tensile behavior of flexural members.

6. CONCLUSIONS

To review the results of the analysis, using outlining column charts for discussion and conclusion

	Beam with f'c=100MPa (20x15)					
	First group Second group)		
samples	B-S2S2	B-G2S2	B-G2G2	B-S3S2	B-G3S2	B-G3G2
Number, kind and size of tensile bars	2steel bar Φ10	2GFRP bar Φ10	2GFRP bar Φ10	3steel bar Φ16	3GFRP bar Φ16	3GFRP bar Φ16
Number, kind and size of compression bars	2steel bar Φ10	2steel bar Φ10	2GFRP bar Φ10	2steel bar Φ10	2steel bar Φ10	2GFRP bar Φ10
Load(n)	38710.26	38710.26	38710.26	38710.26	38710.26	38710.26
Seed size for concrete	0.04	0.04	0.04	0.04	0.04	0.04
Seed size for bar&stirrup	0.06	0.06	0.06	0.06	0.06	0.06
Number of concrete mesh	1000	1000	1000	1000	1000	1000
Number of steel bar mesh	372	372	372	381	381	381
Time period for gravity load (sec)	25	25	25	25	25	25
Time period for main load (sec)	60	50	50	50	50	50
Stress(n/m ²)	1.783E+0 6	1.517E+0 6	4.795E+0 5	1.635E+0 6	1.527E+0 6	4.807E+0 5
Maximum stress in tensile bar.Mises(n/m ²)	1.783E+0 6	5.088E+0 5	4.120E+0 5	1.635E+0 6	5.101E+0 5	4.041E+0 5
Maximum stress in compressive bar.Mises(n/m ²)	1.634E+0 6	1.517E+0 6	3.778E+0 5	1.635E+0 6	1.527E+0 6	3.368E+0 5
Maximum stress in concrete .Mises(n/m ²)	4.650E+0 5	4.789E+0 5	4.795E+0 5	4.684E+0 5	4.700E+0 5	4.807E+0 5
Displacement (m)	5.490E-05	5.570E-05	5.648E-05	5.310E-05	5.530E-05	5.608E-05

Table 7. Summarize the results of the analysis in ABAOUS software



Chart 1. Compares the results of the first group of beams

according to chart (1) and by taking control model (B-S2S2) it shows that after each replace the steel with GFRP bars stage in same condition of loading (ultimate capacity 38710.36 N) stress value is reduce and in the best condition that all of steel bars are replacement , stress value 73% reduced

and according to chart (2) and by taking control model (B-S3S2) it shows that after each replacement the steel with GFRP bars stage in same condition of loading , stress value 70.67% reduced.



Chart 2. Compares the results of the second group of beams

1. In rectangular sections by replacing the tensile bars with GFRP 10 mm bar shows that the average tension 14.85% reduce and if the bars in tensile area increase to 3 bars, the stress 4% reduced. the purpose of study models that in tensile area have 3 longitude bar is ductility that the results of analysis shows 10% increase in ductility.

2. In rectangular sections by replacing all of the steel bars with 10 mm GFRP bars the average stress 75% decrease, by increasing diameter of GFRP bars in tensile area from 10 to 16 m, stress 73.5% decrease and by increasing the number of tensile areas bar from 2 to 3, the average stress 72.67% decrease.

3. beside the all advantage of FRP bars in improving stress behavior of bending concrete reinforced structure , it make s the weight of structures loses.

4. with improving the stress behavior and loses dead load of structure by using GFRP rebars, In future new development in construction industry particularly the tall structures.

5. as the GFRP plomer bars are considered best type of insulation for thermal and magnetic waves , therefore in

special structures such as that hospital and power transmission towers and other strategic structures we can utilize these new materials.

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